



Groundwater Flow Model Report Cimarron Remediation Site

Cimarron Environmental Response Trust

Project No. 142089

Revision 0 10/7/2022

Groundwater Flow Model Report Cimarron Remediation Site

prepared for

Cimarron Environmental Response Trust

Crescent, Oklahoma

Project No. 142089

Revision 0 10/7/2022

prepared by

Burns & McDonnell Engineering Company, Inc. Enter City, State of Office Location

TABLE OF CONTENTS

EXECUTIVE SUMMARY

Page No.

1.0	INTR	ODUCTION
• •	DA4	
2.0	BAT	GROUNDWATER FLOW MODEL CONSTRUCTION
	2.1	Groundwater Model Domain2-1
	2.2	Groundwater Model Discretization
	2.3	Groundwater Model Layering2-3
	2.4	Model Perimeter Boundary Conditions2-3
		2.4.1 No Flow Boundaries
		2.4.2 General Head Boundaries
		2.4.3 River Boundaries
	2.5	Internal Model Boundary Conditions
		2.5.1 Aquifer Recharge
		2.5.2 Groundwater Wells MNW2 Well Package
		2.5.3 Injection and Extraction Trenches 2-7
	26	Hydrogeologic Properties Error! Bookmark not defined.
	2.0	
3.0	BA1	GROUNDWATER FLOW MODEL CALIBRATION
	3.1	Verification of Model After Grid Re-Discretization
	3.2	Simulated versus Observed Groundwater Heads
	3.3	BA1 Model Limitations and Uncertainty
4.0	BA1	REMEDIATION SIMULATIONS
50	WΔΔ	GROUNDWATER FLOW MODEL CONSTRUCTION 5-1
0.0	5.1	Groundwater Model Domain and Discretization 5-1
	5.1	Model Perimeter Boundary Conditions 5-1
	5.4	5.2.1 No Elevy Doundaries 5.1
		5.2.1 No Flow Boundaries
		5.2.2 Constant Head Boundaries
		5.2.3 General Head Boundaries
		5.2.4 River Boundaries
	5.3	Internal Model Boundary Conditions 5-4
		5.3.1 Aquifer Recharge
		5.3.2 Groundwater Extraction Wells and Trenches
	5.4	Hydrogeologic Properties
	14/4 4	
6.0	WAA	GROUNDWATER FLOW WODEL GALIBRATION
	6.2	WAA Model Limitations and Uncertainty
	0.2	WAA MODELEIIIItations and Oncertainty

Ground	water Flow Model Report	Revision 0	Table of Contents
7.0	WAA REMEDIATION SIMU	JLATIONS	7-1
8.0	SUMMARY AND CONCLU	SIONS	
9.0	REFERENCES		9-1
APPE	NDIX A - 2022 BA1 GROUI DISTRIBUTION BY M	NDWATER FLOW MODEL	LITHOLOGY
APPE	NDIX B - ENVIRONMENT	AL SEQUENCE STRATIGE	RAPHY (ESS)
APPE	NDIX C - 2022 WAA GROU DISTRIBUTION BY M	JNDWATER FLOW MODE	EL LITHOLOGY

LIST OF TABLES

Page No.

Table 2-1:	Model Lithology Zones and Hydrogeologic Properties	2-9
Table 3-1:	Model Computed versus Observed Heads	3-3
Table 4-1:	BA1 Model Simulated Rates for Remedial Wells and Trenches	4-2
Table 5-1:	Model Lithology Zones and Hydrogeologic Properties	5-6
Table 6-1:	WAA Model Computed versus Observed Heads	6-2
Table 7-1:	WAA Model Simulated Rates for Remedial Wells and Trenches	7-1

LIST OF FIGURES

Page No.

Figure 2-1:	BA1 Groundwater Model Domain	. 2-2
Figure 2-2:	BA1 Model Perimeter Boundary Conditions	. 2-4
Figure 2-3:	BA1 Model Internal Boundary Conditions	. 2-6
Figure 3-1:	2022 BA1 Calibrated Steady State Model – Modeled vs Observed Heads	
	(feet)	. 3-2
Figure 3-2:	BA1 Calibrated Model Head Results and Residuals	. 3-5
Figure 4-1:	BA1 Site Facility Decommissioning Plan Rev 3 Calculated Heads	. 4-3
Figure 4-2:	BA1 Groundwater Flow Model MODPATH Particle Results	
	Decommissioning Plan Rev 3	. 4-4
Figure 4-3:	BA1 Site Facility Decommissioning Plan Rev 3 MODPATH Results Near	
	Trenches	. 4-5
Figure 5-1:	WAA Groundwater Flow Model Domain	. 5-2
Figure 5-2:	WAA Model Perimeter Boundary Conditions	. 5-3
Figure 5-3:	WAA Model Internal Boundary Conditions	. 5-5
Figure 6-1:	WAA Calibrated Steady State Model - Modeled vs Observed Heads (feet)	. 6-4
Figure 6-2:	WAA Groundwater Flow Model Calibrated Head Field and Residuals	. 6-5
Figure 7-1:	WAA Site Facility Decommissioning Plan Rev 3 Calculated Heads	. 7-2
Figure 7-2:	WAA Site Facility Decommissioning Plan Rev 3 MODPATH Particle	
	Results	. 7-3

LIST OF ABBREVIATIONS

Abbreviation	Term/Phrase/Name
ARM	Absolute Residual Mean
BA1	Burial Area #1
Burns & McDonnell	Burns & McDonnell Engineering Company, Inc.
EPM	Environmental Properties Management LLC
ft	Feet
ft/day	Feet per Day
GHB	MODFLOW General Head Boundary
GPM	Gallon per Minute
К	Hydraulic Conductivity
MNW2	MODFLOW Multi-Node Well package
RMS	Root Mean Square
Site	Cimarron Site
Trust	Cimarron Environmental Response Trust
U>DCGL	Uranium Greater Than the DCGL Area
VDU	Vertical Distribution of Uranium
WAA	Western Alluvial Area

Burns & McDonnell

i

1.0 INTRODUCTION

On behalf of Environmental Property Management LLC (EPM), Trustee for the Cimarron Environmental Response Trust, Burns & McDonnell Engineering Company, Inc. (Burns & McDonnell) submits this 2022 Groundwater Flow Model Report for the Cimarron site (the Site), located at 100 N. Highway 74, Guthrie, Oklahoma. During this report existing groundwater flow models were updated to evaluate groundwater remediation alternatives for the Western Alluvial Area (WA) and Burial Area #1 (BA1) located on the Site.

The WAA and BA1 groundwater models were originally developed in 2006 (ENSR October, 2006), and have been periodically updated to reflect newly available data and various remedial alternatives:

- Groundwater Flow Modeling Report (ENSR October, 2006)
- Groundwater Flow Model Update, (Burns & McDonnell, 2014)
- 2016 Groundwater Flow Model Update (Burns & McDonnell, 2017a).
- 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020)

The purpose of this report is to document the construction, calibration, and remedial alternative simulations of the WAA groundwater flow model and the BA1 groundwater flow model in support of the Site Facility Decommissioning Plan (Revision 3). Consistent with previous iterations of the CERT groundwater models, MODFLOW-2000 (Harbaugh et al., 2000), a three-dimensional, finite difference groundwater flow computer code, was used for the update to the groundwater models. Model construction and the evaluation of model-predicted output were completed using Groundwater Vistas Version 8. Groundwater Vistas® is a pre- and post-processing software package that was used to create standard format MODFLOW file sets from graphically input data. Model outputs were evaluated using Groundwater Vistas®, ArcGIS Pro® (ESRI) and Microsoft Office programs. Groundwater Vistas was used to provide contoured model-predicted results (model predicted heads and drawdown) and numerical data output. Additional data contouring and evaluation was completed using ArcGIS Pro®. All model units for length are in feet, and all model units for time are in days.

2.0 BA1 GROUNDWATER FLOW MODEL CONSTRUCTION

The BA1 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020) was used as the starting point for revisions of the BA1 groundwater flow model documented in this report. The two improvements made to the BA1 2020 Groundwater Flow Model include decreasing the uniform cell size from ten feet to five feet and updates to the distribution of lithology zones. A reduction in cell size was performed to allow for more accurate analysis of groundwater flow near boundary conditions such as infiltration and extraction trenches. The second improvement included updates of lithology zones and associated hydraulic conductivity within the valley of the BA1 transition zone. The Environmental Sequence Stratigraphy (ESS) and Porosity Analysis (Burns & McDonnell, 2018) was used as the basis for an improved representation of varying lithology and specifically the isolated sand channels within the BA1 transition zone.

2.1 Groundwater Model Domain

The same model domain of the BA1 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020) was used for reconstruction of the groundwater flow model in this report (Figure 2-1). The northern extent of the model domain intersects the boundary of the Cimarron River. Groundwater flow is primarily northward toward the Cimarron River. The eastern and western extents of the model domain were developed at adequate distance to limit impact to flow fields within the BA1 transition area. The southern extent of the model boundary was selected to be upgradient of the BA1 transition area and is oriented along an east-west line approximately parallel to the Reservoir #2 dam (ENSR, 2006).



2.2 Groundwater Model Discretization

The BA1 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020) was used as the starting point for revisions to the BA1 groundwater flow model documented in this report. The BA1 2020 Groundwater Flow Model (Burns & McDonnell, 2020) featured a uniform square cell size of ten feet by ten feet. Re-discretization to a smaller uniform square cell size of five feet by five feet was achieved by splitting each ten feet by ten feet MODFLOW cell into four equal cells. The revised grid model domain consists of 340 rows, 340 columns, 12 layers, 1,387,200 total cells, and 1,126,246 active cells. ArcGIS Pro was utilized to assign the properties of the higher-resolution model grid based upon a spatial location match to the attributes of the BA1 2020 Groundwater Flow Model (Burns & McDonnell, 2020). This approach allowed for model re-discretization while maintaining established layer geometry (layer top and bottom elevations), boundary conditions, and hydrogeologic attributes (hydraulic conductivity, porosity) from the active model domain of the BA1 2020 Groundwater Flow Model (Burns & McDonnell, 2020).

2.3 Groundwater Model Layering

Twelve layers are used to simulate the geology of the BA1 area. The upper eight model layers are generally used to simulate the alluvial aquifer, which is approximately 20 feet thick in most of the model domain, and the lower four layers primarily contain bedrock with lower permeability. The model layers are generally uniform with individual layer thicknesses typically between two to three feet. No adjustments were made to the number of layers or model layer elevations within the active model domain during this model update. The original model layering system setup is further described in the 2006 Groundwater Flow Modeling Report (ENSR, 2006).

2.4 Model Perimeter Boundary Conditions

Model perimeter boundary conditions are used to simulate the conceptual flow into and out of the model domain along the outer perimeter of the active model domain. Model perimeter boundary conditions were developed to mirror those implemented in the BA1 2020 Groundwater Flow Model (Burns & McDonnell, 2020) and include the use of no flow cells, the MODFLOW river package, and general head boundaries. The location of model perimeter boundary conditions is illustrated in (Figure 2-2).

2.4.1 No Flow Boundaries

Outside of the active domain are no flow cells that define the western and eastern boundary of the model domain. Starting water levels for all steady-state model solutions were assigned as being one foot below the top of model Layer 1. The high starting water levels allow for the MODFLOW steady-state solution to start cells within the active model domain as saturated and therefore active. Model cells will then remain active unless calculated by MODFLOW to be dry during a final solution.



2.4.2 General Head Boundaries

General Head Boundaries (GHB) were utilized to simulate upgradient flux into the aquifer along the southern extent of the model domain and model Layer 12 consistent with previous the BA1 2020 Groundwater Flow Model (Burns & McDonnell, 2020). The assigned head and conductance terms assigned to general head boundaries within the model are equal to the BA1 2020 Groundwater Flow Model (Burns & McDonnell, 2020).

2.4.3 River Boundaries

The river package was used to simulate the surface water and groundwater interaction of the Cimarron River as a regional groundwater discharge point within model layers 3 through 6. River boundary cells are based upon the location of river cells within the 2020 BA1 Groundwater Flow Model. Values for assigned river heads, boundary conductance, and riverbed elevation were also maintained at those established by the 2020 BA1 Groundwater Flow Model (Burns & McDonnell, 2020).

2.5 Internal Model Boundary Conditions

Internal model boundary conditions are used to simulate internal sources and sinks including recharge, remedial infiltration trenches, remedial extraction trenches, and pumping wells (Figure 2-3).

2.5.1 Aquifer Recharge

Recharge to groundwater is simulated using the MODFLOW recharge package. The recharge package is used to represent the fraction of precipitation that enters the subsurface as rainfall recharge directly to the groundwater table. The model domain is small enough that significant variability in precipitation is not anticipated, therefore recharge is applied uniformly across the model domain. For the steady-state simulation of groundwater flow the recharge package was used to apply a uniform constant recharge rate of 2.4 inches per year (approximately 8% of annual precipitation) consistent with previous steady-state model values (ENSR, 2006) (Burns & McDonnell, 2020).



Y, INC. eospatial\Maps	asakagan:		all Included		0	160	320	
APAN ase\G					Stephen State	SCALE IN F	EET (
NG COA	LEG	END MONITOR WELL IN ALLUVIUM		ACTIVE MODEL DOMAIN	BA1 GROUND	FIGURE 2-3 WATER FLOW UNDARY CONE	MODEL INTERNAL	
ERI	+	MONITOR WELL IN SANDSTONE B		GENERAL HEAD BOUNDARY CELLS	SITE FACIL	ITY DECOMMIS	SSIONING PLAN	
VGINI	. †	MONITOR WELL IN SANDSTONE C		RIVER BOUNDARY CELLS		(REVISION 3)		
LL EP C Clie		MONITOR WELL IN TRANSITION ZONE		NO FLOW BOUNDARIES	N BUR	NS	\gg	
NNE	۲	EXTRACTION WELL/SUMP		TRENCH MODEL CELLS	MS	DONNELL.	environmental properties management, LLC	
McDO	۲	INJECTION WELL	NOT				Rev No: 0	
JS & P ents/E		- GROUNDWATER EXTRACTION TRENCH	MEA	N SEA LEVEL (NORTH AMERICAN VERTICAL	Preparer: BEL	OCKWOOD	Date: 9/20/2022	
3URN Z:\Clie	-	- GROUNDWATER INJECTION TRENCH	DAT	JM OF 1988).	Reviewer: DCL	EMENT	Date: 9/20/2022	
2022 E Palh: 2		BA1 URANIUM CONTOUR (30 UG/L)	2) BA	ASEMAP: GOOGLE EARTH 2017	Coordinate System WGS 1984 Web Mi	t ercator Auxiliary Sph	ere	

2.5.2 Groundwater Wells MNW2 Well Package

The updated groundwater model simulates extraction wells with discrete, short screen intervals, using the Multi-Node Well (MNW2) Package (Konikow, et. al. 2009). In the MNW2 Package, a single well screen can occur at any position within a model layer if the user specifies the elevation of the top and bottom of the well screen. The MNW2 package uses the specified top and bottom of the screen intervals to distribute the prescribed well pumping rate within the cell and to calculate the additional head loss in the pumping well that occurs due to partial penetration effects. All extraction wells simulated in the BA1 model were simulated with ten-foot screen sections.

2.5.3 Injection and Extraction Trenches

The Site Facility Decommissioning Plan (Revision 3) includes several proposed groundwater injection and extraction trenches. Injection and extraction trenches were simulated utilizing the MODFLOW well package by assigning individual well boundary conditions to model cells which overlapped the linear extent of each infiltration or extraction trench. Injection or extraction rates were then assigned to individual cells based upon the total simulated flow rate for the trench, divided by the number of cells in the well package simulating each trench.

2.6 Injection and Extraction Trenches

The hydrogeologic properties specified within the model are horizontal hydraulic conductivity (K_{xy}), vertical hydraulic conductivity (K_z), and porosity. All modeling simulations were run under steady-state conditions, which do not require specification of aquifer storage coefficients (specific storage or specific yield).

Hydraulic conductivity is a measure of a material's capacity to transmit water and is defined as a constant of proportionality relating the specific discharge of a porous medium under a unit hydraulic gradient. The units for hydraulic conductivity within this report are provided as feet per day (feet/day). Hydraulic conductivity values are required to describe the permeability of each cell in the MODFLOW model. The BA1 model represents a complicated layering system of unconsolidated deposits underlain by semi-permeable bedrock (ENSR, 2006). The distribution of hydraulic conductivity within the model is based upon hydraulic conductivity zones which correlate to a specific lithology type.

Distribution of hydraulic conductivity values for this model update began with utilizing values established by the 2020 BA1 Groundwater Flow Model (Burns & McDonnell, 2020). The intended use of this model update is additional examination of groundwater flow and transport conditions, specifically within the BA1 transition zone under the remedial conditions of the Site Facility Decommissioning Plan (Revision 3). In 2018 an ESS and porosity analysis (Burns & McDonnell, 2018) was performed which developed a high-resolution three-dimensional interpretation of the lithology within the BA1 transition zone (Appendix B). This ESS analysis included three-dimensional interpolation of specific lithology zones which include:

- Cimarron River Floodplain Deposits Clay, silt, and interbedded fine-grained sand corresponding to floodplain deposits of the Cimarron River. Includes sands as overbank splays deposited during flood-stages.
- Cimarron River Channel Deposits Fine to coarse grained, cross-bedded sand deposited as point-bars by the Cimarron River.
- Cimarron River Clay Plug Deposits: Clay and silt with some thin sands, deposited in abandoned stretches of Cimarron River channels (oxbow lakes).
- Upper Gully Fill Silt and silty sand with interbedded clayey sand and silty sand deposited as gully-wash by streamflow during flash flood events. Contains minor sand-rich streamflow deposits.
- Lower Gully Fill: Clay-rich deposits including gully-wall failure (slump, slide, and debris-flow) features. Chaotic, may include minor re-worked streamflow deposits.
- Intra-gully Stream Deposits: Sand and silty sand deposited by streamflow within gully system.
- Garber Sandstone Bedrock (undifferentiated).

Using ArcGIS Pro® and Groundwater Vistas 8®, the three-dimensional distribution of lithology zones within the ESS model was incorporated into the groundwater model using a nearest neighbor merge of the MODFLOW cell nodes to the three-dimensional ESS lithology coverage. Each of the unconsolidated sediment lithologies defined by ESS was assigned a distinct zone within the model so that model hydraulic conductivity and porosity attributes are grouped by lithology zone. The final distribution of lithology zones is provided within Table 2-1 and illustrated within Appendix A.

Lithology	Groundwater Model Lithology Zone Number	K _x	Ky	Kz	Porosity
Cimarron River Floodplain Deposits Clay/Silt traces sand	101	3	3	0.3	0.2
Cimarron River Deposits - Upper Alluvial Aquifer Sands	2 and 102	117.5	117.5	11.75	0.3
Cimarron River Deposits - Lower Alluvial Aquifer Sands	12	352.5	352.5	35.25	0.3
River Clay Plug Deposits	103	2.77	2.77	0.277	0.2
Uppermost Gulley Fill Unit	5	1.28	1.28	0.128	0.2
Upper Gully Fill	104	15	15	1.5	0.2
Lower Gully Fill	105	3	3	0.3	0.2
Intra Gully Stream Deposits (Sand Body)	106	50	50	5	0.3
Clay	10	0.5	0.5	0.05	0.2
Silt	3	0.283	0.283	0.0283	0.2
Siltstone	6, 8, and 9	8.43	8.43	0.422	0.01
Sandstone A	4	40	40	2	0.05
Sandstone B	7	5	5	0.25	0.05
Sandstone C	11	3	3	0.15	0.05

TILAA	NA 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
Table 2-1:	Wodel Lithology Zones and Hydrogeolog	JIC Properties

3.0 BA1 GROUNDWATER FLOW MODEL CALIBRATION

After updating model discretization and lithology zones, validation of model calibration was evaluated by comparing observed and simulated groundwater elevations, groundwater flow contours, and water budgets. The calibration goals for the numerical model are based upon industry standards and previous BA1 modeling efforts which are defined as:

- A less than one (1) percent water balance error, which is considered appropriate for a calibrated groundwater model (Anderson and Woessner, 1992). The water balance error is defined as the total inflow minus the total outflow, divided by either the inflow or outflow, whichever yields the highest error.
- A Normalized Root Mean Square error (NRMS) of less than ten (10) percent. A NRMS of less than ten (10) percent is generally considered appropriate for a calibrated groundwater model (Anderson and Woessner, 1992). A lower NRMS indicates a better statistical model calibration.
- An Absolute Residual Mean (ARM) of less than ten percent the observed head change value across the model domain. The ARM can be described as the average error of the absolute value of the residuals.
- A qualitative match of model simulated potentiometric surface and observed potentiometric surface, evaluated by visually comparing contours. When calibrated, the model should be able to reproduce the direction and magnitude of the hydraulic gradient observed within the boundary.

3.1 Verification of Model After Grid Re-Discretization

The groundwater flow model was updated to a smaller uniform square grid cell size of five feet by five feet. Prior to any other model changes, the calculated groundwater heads from the updated grid model were then compared to the heads obtained from the 2020 groundwater flow model. The comparison found that the heads in the updated grid model and the 2020 groundwater flow model were nearly identical. The near identical heads confirm that model grid refinement and the re-import of model attributes did not significantly influence model calibration.

3.2 Simulated versus Observed Groundwater Heads

As documented in the model construction portion of this report, updates to the distribution of lithology zones were completed after refinement of the groundwater model grid. After completing the modifications to the distribution of lithology zones, water level measurements collected in August 2016 were compared to the model calculated head values. The calibration data set and the model calculated heads reflect non-pumping conditions prior to implementation of any remedial alternatives. The

Revision 0

BA1 Groundwater Flow Model Calibration

calibration dataset included 68 wells with a range in observed water level elevations of 17.48 feet. The model simulated heads and observed heads used for the calibration dataset are within Table 3-1.

The calibration statistics for the updated model indicate a mass balance error of 0.0012 percent, NRMS of 0.059 (5.9 percent), and ARM of 0.64 feet which meet the established model calibration goals. As an additional evaluation for the model calibration, the simulated versus observed groundwater level data for the calibrated steady state model is provided as Figure 3-1 and indicates a good fit between simulated and observed head data. The resulting flow field of the calibrated groundwater model and distribution of residuals are illustrated Figure 3-2. The updated model calibration is an improvement of the calibration statistics from the 2016 Groundwater Flow Model Update which achieved a NRMS of 0.069 (6.9 percent) and ARM of 0.7 feet (Burns & McDonnell, 2017a).



Figure 3-1: 2022 BA1 Calibrated Steady State Model – Modeled vs Observed Heads (feet)

Revision 0

BA1 Groundwater Flow Model Calibration

Observation	x	Y	Model	Observed	Computed	Residual
Well Name	Coordinate	Coordinate	Layer	Head (ft)	Head (ft)	(ft)
02W02	2095455	322885	6	930.53	930.93	-0.40
02W03	2095375	322885	5	928.42	929.94	-1.52
02W04	2095335	322905	6	927.88	928.24	-0.36
02W05	2095315	322955	5	927.88	928.01	-0.13
02W06	2095305	323005	7	927.87	927.85	0.02
02W07	2095345	323005	7	927.87	927.84	0.03
02W08	2095395	323015	7	927.85	927.78	0.07
02W09	2095595	322765	6	935.13	935.43	-0.30
02W10	2095575	322825	6	933.81	933.69	0.12
02W11	2095445	323055	8	927.74	927.65	0.09
02W12	2095455	323035	8	927.73	927.68	0.05
02W13	2095475	322985	8	927.93	927.81	0.12
02W14	2095395	323055	8	927.76	927.68	0.08
02W15	2095285	322895	5	927.91	928.24	-0.33
02W16	2095265	322945	6	927.90	928.06	-0.16
02W17	2095255	323005	7	927.86	927.87	-0.01
02W18	2095345	323095	8	927.74	927.61	0.13
02W19	2095325	323055	7	927.82	927.71	0.11
02W21	2095195	323055	8	928.41	927.75	0.66
02W22	2095215	322935	6	927.89	928.10	-0.21
02W23	2095205	323005	8	927.89	927.88	0.01
02W24	2095265	323055	8	927.83	927.73	0.10
02W26	2095625	322715	5	935.88	936.73	-0.85
02W27	2095395	322825	6	932.18	932.72	-0.54
02W28	2095535	322835	6	933.91	933.07	0.84
02W29	2095555	322755	5	934.99	935.68	-0.69
02W30	2095475	322765	7	934.91	935.52	-0.61
02W31	2095505	322855	6	933.53	932.45	1.08
02W32	2095435	322965	7	927.87	927.90	-0.03
02W33	2095255	322915	6	927.96	928.16	-0.20
02W34	2095185	323105	8	927.84	927.64	0.20
02W35	2095255	323155	8	927.75	927.53	0.22
02W36	2095255	323105	8	927.78	927.62	0.16
02W37	2095325	323155	7	927.69	927.51	0.18
02W38	2095395	323095	8	927.70	927.59	0.11
02W39	2095575	322735	5	935.29	936.32	-1.03
02W40	2095525	322665	7	939.37	939.49	-0.12
02W41	2095575	322685	6	937.77	938.13	-0.36

Table 3-1: Model Computed versus Observed Heads

Revision 0

BA1 Groundwater Flow Model Calibration

Observation	X	Y	Model	Observed	Computed	Residual
Well Name	Coordinate	Coordinate	Layer	Head (ft)	Head (ft)	(ft)
02W42	2095475	322725	7	937.06	937.45	-0.39
02W43	2095325	323205	8	927.66	927.43	0.23
02W44	2095375	323155	8	927.65	927.49	0.16
02W45	2095285	323195	8	927.69	927.46	0.23
02W46	2095465	322905	6	929.07	930.29	-1.22
02W47	2095525	322625	7	940.39	940.92	-0.53
02W50	2095525	322565	7	940.91	942.77	-1.86
02W52	2095555	322565	7	940.25	942.09	-1.84
02W53	2095385	322825	6	932.28	932.68	-0.40
02W62	2095205	323145	8	927.77	927.56	0.21
1314	2095465	322415	8	944.45	947.88	-3.43
1344	2095775	323505	7	926.97	927.05	-0.08
1361	2095435	323265	8	927.53	927.31	0.22
1362	2095455	323185	10	927.61	927.08	0.53
1315R	2095505	322755	7	934.62	935.87	-1.25
1316R	2095435	322775	7	933.38	935.21	-1.83
TMW-01	2095505	322695	7	942.72	938.53	4.19
TMW-02	2095505	322595	7	940.77	942.31	-1.54
TMW-05	2095555	322885	7	932.30	931.92	0.38
TMW-06	2095635	322795	4	934.64	934.80	-0.16
TMW-08	2095535	322725	6	935.37	936.89	-1.52
TMW-09	2095485	322825	6	933.65	933.19	0.46
TMW-13	2095375	322955	6	927.90	927.96	-0.06
TMW-17	2095495	322765	12	932.22	934.50	-2.28
TMW-18	2095335	322865	6	928.12	929.89	-1.77
TMW-19	2095335	322865	4	928.99	930.06	-1.07
TMW-21	2095435	322705	6	937.22	938.66	-1.44
TMW-24	2095435	323405	7	927.44	927.17	0.27
TMW-25	2095625	322655	5	937.22	938.57	-1.35

3.3 BA1 Model Limitations and Uncertainty

All models are a simplified representation of the physical aquifer system. Use of the updated groundwater model documented in this report is appropriate for the development of the conclusions provided within this report. Site conditions and hydrogeologic properties have been estimated through extrapolation of measured or estimated properties based on existing site information and professional judgment. Use of the groundwater model is currently limited to steady-state analyses which are intended to represent long-term static groundwater elevations or specific remedial alternatives. Additional specification of aquifer storage terms would be required for implementation of transient MODFLOW solutions.





4.0 BA1 REMEDIATION SIMULATIONS

For this groundwater model update, particle tracking was completed under the nominal extraction and injection rates proposed in the current BA1 Site Facility Decommissioning Plan (Revision 3). Infiltration trench GWI-BA1-04 was added to address the potential for dewatering of the coarse grained intra gully sand deposits between GETR-BA1-01 and GETR-BA1-02. Implementing infiltration trench GWI-BA1-04 will raise groundwater levels and provides additional flushing of the pore space in the unconsolidated sediments between GETR-BA1-01 and GETR-BA1-02. The nominal rates used to simulate the extraction and injection infrastructure within the model are summarized in Table 4-1.

A simulated flow rate of ten gallons per minute (gpm) was selected for GW1-BA1-04. This flow rate was determined based upon iteratively increasing the flow rate to GWI-BA1-04 until achieving near zero drawdown across the extent of the sands of the intra-gully stream deposits within model Layer 7. The resulting groundwater heads for the steady-state MODFLOW solution based upon the injection and extraction rates within Table 4-1 are illustrated within Figure 4-1.

The groundwater heads and cell flux information from the MODFLOW solution were then input into a 30-year MODPATH particle tracking simulation (Pollock, 1989). MODPATH utilizes the results of the MODFLOW model along with specified porosity values and user-specified starting particle locations to calculate a three-dimensional pathline. Particles are tracked individually through the simulated flow system using the calculated distribution of velocity throughout the flow system. MODPATH was selected for this modeling study because of its applicability and simple linkage with MODFLOW. Particles were placed in or near each cell representing an injection trench and near the outer boundaries of the uranium plume. MODPATH particle tracking results for the BA1 uranium plume remediation area is presented in Figure 4-2. Particle tracking results near infiltration trench GWI-BA1-04 indicate that particles are either captured by the adjacent infiltration trench or flushed to the nearest downgradient extraction well (Figure 4-3).

Revision 0

Trench or Well Name	Extraction or Injection	Extraction or Injection Rate (GPM)
GWI-BA1-01	Injection Trench	10
GWI-BA1-02	Injection Trench	4
GWI-BA1-03	Injection Trench	4
GWI-BA1-04	Injection Trench	10
GETR-BA1-01	Extraction Trench	7
GETR-BA1-02	Extraction Trench	7
GE-BA1-02	Extraction Well	31
GE-BA1-03	Extraction Well	24
GE-BA1-04	Extraction Well	31

Table 4-1: BA1 Model Simulated Rates for Remedial Wells and Trenches





VIUM STONE B STONE C	PUMPING HEAD CONTOURS (1 FT) BA1 URANIUM CONTOUR (30 UG/L) GENERAL HEAD BOUNDARY CELLS	FIGURE 4-2 BA1 GROUNDWATER FL MODPATH PARTICLE TRAC SITE FACILITY DECOMMIS (REVISION 3	OW MODEL KING RESULTS SIONING PLAN)
SITION ZONE	RIVER BOUNDARY CELLS NO FLOW BOUNDARIES TRENCH MODEL CELLS		environmental properties management, LLC
TION TRENCH	£		<u>Rev No:</u> 0
ON TRENCH	NOTES	Preparer: BELOCKWOOD	Date: 9/20/2022
ON ARROWS	LEVEL (NORTH AMERICAN VERTICAL DATUM OF 1988).	Reviewer: DCLEMENT	Date: 9/20/2022
	2) BASEMAP: GOOGLE EARTH 2017	<u>Coordinate System</u> WGS 1984 Web Mercator Auxiliary Sphe	re



GROUNDWATER INJECTION TRENCH

BA1 URANIUM CONTOUR (30 UG/L)

1988).

2) BASEMAP: GOOGLE EARTH 2017

	946		SCALE IN F	AET A
LEG	END	FIGURE 4-3		
	MONITOR WELL IN ALLUVIUM	PARTICLE TRACKING	BA1 GROUNDWATER F	LOW MODEL
+	MONITOR WELL IN SANDSTONE B	> PARTICLE FLOW DIRECTION ARROWS	NEAR TRENCH	HES
+	MONITOR WELL IN SANDSTONE C	PUMPING HEAD CONTOURS (1 FT)	SITE FACILITY DECOMMISSIONING	3 PLAN (REVISION 3)
	MONITOR WELL IN TRANSITION ZONE	PUMPING HEAD CONTOURS (0.5 FT)	BURNS	\gg
•	EXTRACTION WELL/SUMP	TRENCH MODEL CELLS	MSDONNELL.	environmental properties management, LLC
٥	INJECTION WELL			Rev No: 0
	- GROUNDWATER EXTRACTION TRENCH	NOTES 1) GROUNDWATER ELEVATIONS IN FEET ABOVE MEAN	Preparer: BELOCKWOOD	Date: 9/20/2022
		SEA LEVEL (NORTH AMERICAN VERTICAL DATUM OF	Deviewer DOLEMENT	Data: 0/00/0000

Reviewer: DCLEMENT

Coordinate System WGS 1984 Web Mercator Auxiliary Sphere

9/20/2022

Date:

Revision 0

5.0 WAA GROUNDWATER FLOW MODEL CONSTRUCTION

The WAA Groundwater Flow Model described by (Burns & McDonnell, 2020) was used as the model for remedial alternative simulations. The sections below document model construction.

5.1 Groundwater Model Domain and Discretization

The same model domain of the 2020 Groundwater Flow Model Update (Burns & McDonnell, 2020) was used for construction of the groundwater flow model in this report. The model domain for the WA area was set up to include the area from the escarpment to the south to the Cimarron River to the north and east and west to distances to have a negligible effect on groundwater flow conditions within the interior of the model domain (Figure 5-1). The model was developed with 402 rows, 412 columns, and three layers for which grid cells are approximately 10 feet square in the X-Y plane. This results in 496,872 total cells with 407,245 cells within the active model domain.

5.2 Model Perimeter Boundary Conditions

Model perimeter boundary conditions are used to simulate the conceptual flow into and out of the model domain along the outer perimeter of the active model domain. Model perimeter boundary conditions are the same as those described by the 2020 Groundwater Flow Model (Burns & McDonnell, 2020) and include the use of no flow cells, the MODFLOW river package, and general head boundaries. The location of model perimeter boundary conditions is illustrated in (Figure 5-2).

5.2.1 No Flow Boundaries

Outside of the active domain are no flow cells that define the western and eastern boundary of the entire model domain. Starting water levels for all steady-state model solutions were assigned as being one foot below the top of model Layer 1. The high starting water levels allow for the MODFLOW steady-state solutions to start cells within the active model domain as saturated and therefore active. Model cells will then remain active unless calculated by MODFLOW to be dry during a final solution.

5.2.2 Constant Head Boundaries

The impact of leakage to groundwater from Reservoir 3 on the groundwater elevations within the WAA model is simulated utilizing a coverage of constant head boundary cells. The constant head boundaries are assigned an elevation of 958 feet msl. The assigned head elevation based on prior investigations (Burns & McDonnell, 2017a).



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figure 1 - GW Model Domain

FIGURE 5-1 WAA GROUNDWATER FLOW MODEL DOMAIN FACILITY DECOMMISSIONING PLAN **REVISION 3** environmental LEGEND MONITOR WELL IN ALLUVIUM MONITOR WELL IN SANDSTONE A MONITOR WELL IN SANDSTONE B MONITOR WELL IN SANDSTONE C MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN NOTES 1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988). 550 1,100 SCALE IN FEET Rev No: 0 Date: 10/6/2022 Preparer: DHORNE Reviewer: DCLEMENT Date: 10/6/2022 Coordinate System NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figure 2 - Perimeter Boundary Conditions

FIGURE 5-2 WAA GROUNDWATER FLOW MODEL PERIMETER BOUNDARY CONDITIONS FACILITY DECOMMISSIONING PLAN **REVISION 3** environmental procerties management, the LEGEND MONITOR WELL IN ALLUVIUM MONITOR WELL IN SANDSTONE A MONITOR WELL IN SANDSTONE B MONITOR WELL IN SANDSTONE C MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN NO FLOW BOUNDARIES CONSTANT HEAD BOUNDARIES **RIVER BOUNDARIES CELLS** GENERAL HEAD BOUNDARY CELLS NOTES 1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988). 550 0 1,100 SCALE IN FEET Rev No: 0 Preparer: DHORNE Date: 10/6/2022 Reviewer: DCLEMENT Date: 10/6/2022 Coordinate System NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet

Revision 0

5.2.3 General Head Boundaries

General Head Boundaries (GHB) were utilized to simulate upgradient flux into the aquifer along the southern extent of the model domain (Layer 2), and flux from underlying bedrock (Layer 3). The locations, assigned heads, and conductance terms allocated to general head boundaries within the model are equal to the 2020 Groundwater Flow Model (Burns & McDonnell, 2020).

5.2.4 River Boundaries

The river package was used to simulate the surface water and groundwater interaction of the Cimarron River as a regional groundwater discharge point within model Layers 1 and 2. River boundary cells are based upon the location of river cells within the 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020). Values for assigned river heads, boundary conductance, and riverbed elevation were also maintained at those established by the 2020 Groundwater Flow Model Review.

5.3 Internal Model Boundary Conditions

Internal model boundary conditions are used to simulate internal sources and sinks including recharge, remedial infiltration trenches, remedial extraction trenches, and pumping wells (Figure 5-3).

5.3.1 Aquifer Recharge

Recharge to groundwater is simulated using the MODFLOW recharge package. The recharge package is used to represent the fraction of precipitation that enters the subsurface as rainfall recharge directly to the groundwater table. The model domain is small enough that significant variability in precipitation is not anticipated, therefore recharge is applied uniformly across the model domain. For the steady-state simulation of groundwater flow the recharge package was used to apply a uniform constant recharge rate of 2.4 inches per year (approximately 8% of annual precipitation) consistent with previous steady-state model values (ENSR, 2006) (Burns & McDonnell, 2020).

5.3.2 Groundwater Extraction Wells and Trenches

The Site Facility Decommissioning Plan (Revision 3) includes several proposed groundwater extraction wells and trenches. The WAA groundwater model simulates extraction wells, extraction trenches, and injection trenches utilizing the MODFLOW well package. Extraction trench GETR-WU-01A was simulated utilizing the MODFLOW well package by assigning individual well boundary conditions to model cells which overlapped the linear extent of the trench. Flux from infiltration trench GWI-WU-01A reaching the end of nearby interceptor trench and piping is simulated as a group of MODFLOW well package cells near the downhill termination of the interceptor collection.



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figure 3 - Internal Boundary Conditions

	FIGURE 5-3 WAA GROUNDWATER FLOW MODEL INTERNAL BOUNDARY CONDITIONS FACILITY DECOMMISSIONING PLAN REVISION 3	
	BURNS MSDONNELL. environmental popenies mailinguitant.tcc	
	LEGEND MONITOR WELL IN ALLUVIUM MONITOR WELL IN SANDSTONE A MONITOR WELL IN SANDSTONE B MONITOR WELL IN SANDSTONE C MONITOR WELL IN TRANSITION ZONE EXTRACTION WELL INJECTION WELL	
	GROUNDWATER EXTRACTION TRENCH GROUNDWATER INJECTION TRENCH ACTIVE MODEL DOMAIN NO FLOW BOUNDARIES RIVER BOUNDARY CELLS CONSTANT HEAD BOUNDARIES GENERAL HEAD BOUNDARY CELLS	×
- Bark -		
1 2		
	<u>NOTES</u> 1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988).	*
	0 550 1,100 N SCALE IN FEET	
35.4	Rev No: 0	
-	Preparer: DHORNE Date: 10/6/2022	
-	Reviewer: DCLEMENT Data: 10/6/2022	
	Coordinate System	
1	NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet	

5.4 Hydrogeologic Properties

The hydrogeologic properties specified within the model are horizontal hydraulic conductivity (K_{xy}), vertical hydraulic conductivity (K_z), and porosity. All modeling simulations were run under steady-state conditions, which do not require specification of aquifer storage coefficients (specific storage or specific yield). The WAA model represents a layering system of unconsolidated deposits underlain by semipermeable bedrock (ENSR, 2006). The distribution of hydraulic conductivity within the model is based upon hydraulic conductivity zones which correlate to a specific lithology type. Distribution of hydraulic conductivity is based upon values established by the 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020). The final distribution of lithology zones is provided within Table 5-1 and illustrated within Appendix C.

Lithology	Groundwater Model Lithology Zone Number	K _x	Ky	Kz	Porosity
Cimarron River Deposits Upper Alluvial Aquifer Sands	5	117.5	117.5	11.75	0.3
Cimarron River Deposits Lower Alluvial Aquifer Sands	2	117.5	117.5	11.75	0.3
Sandstone	4	3	3	0.15	0.05

Table 5-1: Model Lithology Zones and Hydrogeologic Properties

Revision 0

6.0 WAA GROUNDWATER FLOW MODEL CALIBRATION

Validation of the WAA model calibration was evaluated by comparing observed and simulated groundwater elevations, groundwater flow contours, and water budgets. The calibration goals for the numerical model are based upon industry standards and previous WAA modeling efforts which are defined as:

- A less than one (1) percent water balance error, which is considered appropriate for a calibrated groundwater model (Anderson and Woessner, 1992).
- A Normalized Root Mean Square error (NRMS) of less than ten (10) percent.
- An Absolute Residual Mean (ARM) of less than ten percent the observed head change value across the model domain.
- A qualitative match of model simulated potentiometric surface and observed potentiometric surface, evaluated by visually comparing contours.

6.1 Simulated versus Observed Groundwater Heads

Water level measurements collected in August 2016 were compared to the model calculated head values as part of model calibration. The calibration data set and the model calculated heads reflect non-pumping conditions prior to implementation of any remedial alternatives. The calibration dataset included 70 wells with a range in observed water level elevations of 26.03 feet.

The model simulated heads and observed heads used for the calibration dataset are within Table 6-1. The calibration statistics for the WAA model indicate a mass balance error of 0.0034 percent, NRMS of 0.033 (3.3 percent), and ARM of 0.62 feet which meet established model calibration goals. As an additional evaluation for the model calibration, the simulated versus observed groundwater level data for the calibrated steady state model is provided as Figure 6-1 and indicates a good fit between simulated and observed head data. The resulting flow field of the calibrated groundwater model and distribution of residuals are illustrated Figure 6-2.

WAA Groundwater Flow Model Calibration

Observation	X	Y	Observed	Computed	Residual
Well Name	Coordinate	Coordinate	Head (ft)	Head (ft)	(ft)
T-51	2091962	322775	929.40	929.59	-0.19
T-52	2092407	321938	929.33	929.99	-0.66
T-53	2092659	322773	929.20	929.45	-0.26
T-54	2092871	321928	929.90	929.89	0.01
T-55	2093120	322070	928.46	929.74	-1.28
T-56	2093378	322211	927.75	929.61	-1.86
T-57	2092461	321788	930.23	930.05	0.18
T-58	2092165	321742	930.42	930.13	0.29
T-59	2092955	322774	929.18	929.40	-0.22
T-60	2093282	322774	929.20	929.36	-0.16
T-61	2093610	322774	929.03	929.34	-0.31
T-62	2091853	321471	930.69	930.28	0.41
T-63	2091977	321623	930.50	930.20	0.30
T-64	2091691	321342	930.85	930.53	0.32
T-65	2091814	321569	930.65	930.24	0.41
T-66	2091842	321712	930.53	930.19	0.34
T-67	2091743	321657	930.61	930.22	0.39
T-68	2091713	322052	930.25	930.04	0.20
T-69	2091872	321962	930.35	930.07	0.27
T-70R	2091626	321578	930.72	930.26	0.46
T-72	2091717	321899	930.40	930.12	0.28
T-73	2091492	321771	930.53	930.19	0.34
T-74	2091531	321541	930.80	930.28	0.52
T-75	2091598	321911	930.08	930.12	-0.04
T-76	2091731	321776	930.52	930.17	0.34
T-77	2091578	322010	930.29	930.08	0.21
T-78	2091494	321897	930.39	930.14	0.25
T-79	2091582	322213	930.07	929.97	0.10
T-81	2091476	321994	930.29	930.09	0.20
T-82	2091569	322414	931.77	929.86	1.91
T-83	2091501	322297	929.80	929.93	-0.13
T-84	2091869	322295	929.92	929.89	0.03
T-85	2092243	322346	929.81	929.79	0.01
T-86	2092647	322374	929.63	929.69	-0.06
T-87	2092979	322422	929.40	929.58	-0.19
T-88	2093384	322464	929.10	929.50	-0.40
T-89	2093072	323042	928.73	929.22	-0.49
T-90	2092830	323042	928.85	929.25	-0.40

Table 6-1: WAA Model Computed versus Observed Heads

•

Revision 0

WAA Groundwater Flow Model Calibration

. . ..

Observation	X	Y	Observed	Computed	Residual
Well Name	Coordinate	Coordinate	Head (ft)	Head (ft)	(ft)
T-91	2092966	323228	927.63	929.10	-1.48
T-92R	2093121	323143	925.85	929.15	-3.30
T-93	2093414	323104	928.66	929.16	-0.50
T-94	2093267	323409	928.31	928.95	-0.64
T-95	2092458	323019	928.98	929.34	-0.36
T-96	2091985	322557	929.56	929.72	-0.16
T-97	2092039	323318	928.78	929.20	-0.42
T-98	2092176	323514	928.61	929.03	-0.42
T-99	2092590	323746	928.25	928.79	-0.54
T-100	2093060	323821	927.05	928.54	-1.49
T-101	2093508	323599	927.99	928.84	-0.85
T-102	2093581	323085	928.69	929.17	-0.48
T-103	2094028	322867	928.86	929.33	-0.47
1319B-1	2092053	320128	947.62	946.62	0.99
1319B-2	2092078	320000	948.71	947.85	0.86
1319B-3	2092005	320105	947.82	946.51	1.31
1319B-4	2092053	320207	947.11	946.01	1.10
1319B-5	2091860	320322	945.37	943.99	1.38
1338	2093546	321819	944.27	943.25	1.02
1341	2092542	321355	937.68	937.36	0.33
1345	2092347	321461	934.66	933.99	0.67
1346	2093200	321854	938.38	936.47	1.91
1382	2093128	321736	938.76	937.56	1.20
1384	2093399	321602	945.03	944.25	0.78
1386	2093376	321918	939.89	938.00	1.89
1388	2093710	321837	946.55	946.73	-0.18
1390	2093720	322017	942.47	942.17	0.30
1391	2093820	321752	951.88	951.98	-0.10
1392	2093115	321861	936.82	934.88	1.94


Revision 0



Figure 6-1: WAA Calibrated Steady State Model - Modeled vs Observed Heads (feet)

6.2 WAA Model Limitations and Uncertainty

All models are a simplified representation of the physical aquifer system. Use of the updated groundwater model documented in this report is appropriate for the development of the conclusions provided within this report. Site conditions and hydrogeologic properties have been estimated through extrapolation of measured or estimated properties based on existing site information and professional judgment. Use of the groundwater model is currently limited to steady-state analyses which are intended to represent long-term static groundwater elevations or specific remedial alternatives. Additional specification of aquifer storage terms would be required for implementation of transient MODFLOW solutions.



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figure 7 - Calibration Model

FIGURE 6-2 WAA GROUNDWATER FLOW MODEL CALIBRATED HEADS / REDIDUALS FACILITY DECOMMISSIONIONG PLAN (REVISION3) environmental LEGEND MONITOR WELL IN ALLUVIUM MONITOR WELL IN SANDSTONE A MONITOR WELL IN SANDSTONE B MONITOR WELL IN SANDSTONE C MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN NO FLOW BOUNDARIES CONSTANT HEAD BOUNDARIES RIVER BOUNDARIES CELLS GENERAL HEAD BOUNDARY CELLS - CALIBRATION HEAD CONTOURS (FEET)

NOTES

1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988).

0 550 1 SCALE IN FEET	,100
	<u>Rev No:</u> 0
Preparer: DHORNE	Date: 10/6/2022
Reviewer: DCLEMENT	Date: 10/6/2022
Coordinate System NAD 1983 StatePlane Oklahoma North F	FIPS 3501 Feet

Groundwater Flow Model Report Revision 0 WAA Remediation Simulations

7.0 WAA REMEDIATION SIMULATIONS

For this groundwater model update, particle tracking was completed under the nominal extraction and injection rates proposed in the current Site Facility Decommissioning Plan (Revision 3). The nominal rates used to simulate the extraction and injection infrastructure within the model are summarized in Table 7-1. The resulting groundwater heads for the steady-state MODFLOW solution based upon the injection and extraction rates within Table 7-1 are illustrated within Figure 7-1.

Trench or Well Name	Extraction or Injection	Extraction or Injection Rate (GPM)
GE-WAA-04	Extraction Well	20
GE-WAA-05	Extraction Well	25
GE-WAA-02	Extraction Well	30
GE-WAA-03	Extraction Well	24
GETR-WU-01A	Extraction Trench	8
GWI-WU-01	Infiltration Trench	8

Table 7-1: WAA Model Simulated Rates for Remedial Wells and Trenches

The groundwater heads and cell flux information from the MODFLOW solution were then input into a 30-year MODPATH particle tracking simulation (Pollock, 1989). MODPATH utilizes the results of the MODFLOW model along with specified porosity values and user-specified starting particle locations to calculate a three-dimensional pathline. Particles are tracked individually through the simulated flow system using the calculated distribution of velocity throughout the flow system. MODPATH was selected for this modeling study because of its applicability and simple linkage with MODFLOW. Particles were placed near the outer boundaries of the remediation area. MODPATH particle tracking results for the remediation area is presented in Figure 7-2. Particle tracking indicate that all particles are captured by the proposed extraction wells.



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figure 8 - DPlan Rev 3 Head Results

FIGURE 7-1 WAA SITE FACILITY DECOMMISSIONING PLAN (REV 3) MODEL CALCULATED HEADS



LEGEND

- MONITOR WELL IN ALLUVIUM
- MONITOR WELL IN SANDSTONE A
- MONITOR WELL IN SANDSTONE B
- + MONITOR WELL IN SANDSTONE C
- MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN
- NO FLOW BOUNDARIES
- CONSTANT HEAD BOUNDARIES
- --- WA 2022 Nominal Q Head Contours
- RIVER BOUNDARY CELLS
- GENERAL HEAD BOUNDARY CELLS

NOTES

1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988).

0 550 SCALE IN FEET	
	<u>Rev No:</u> 0
Preparer: DHORNE	Date: 10/6/2022
Reviewer: DCLEMENT	Date: 10/6/2022
Coordinate System NAD 1983 StatePlane Oklahoma North	FIPS 3501 Feet



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figure 10 - DPlan Rev 3 Modpath Results Pumping

FIGURE 7-2 WAA SITE FACILITY DECOMMISSIONING PLAN (REV 3) MODPATH PARTICLE TRACKING RESULTS BURNS MSDONNELL. environmental properties management.uc LEGEND MONITOR WELL IN ALLUVIUM MONITOR WELL IN SANDSTONE A MONITOR WELL IN SANDSTONE B MONITOR WELL IN SANDSTONE C MONITOR WELL IN TRANSITION ZONE EXTRACTION WELL INJECTION WELL GROUNDWATER EXTRACTION TRENCH - GROUNDWATER INJECTION TRENCH ACTIVE MODEL DOMAIN NO FLOW BOUNDARIES CONSTANT HEAD BOUNDARIES - NOMINAL HEAD CONTOURS RIVER BOUNDARY CELLS GENERAL HEAD BOUNDARY CELLS - PARTICLE TRACKING -> PARTICLE FLOW DIRECTION ARROWS WELL PACKAGE CELLS NOTES 1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988). 130 260 SCALE IN FEET Rev No: 0 Date: 10/6/2022 Preparer: DHORNE Reviewer: DCLEMENT Date: 10/6/2022 Coordinate System NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet

Revision 0

8.0 SUMMARY AND CONCLUSIONS

This groundwater flow model report documents the construction, calibration, and remedial alternative simulations of the WAA groundwater flow model and the BA1 groundwater flow model in support of the Site Facility Decommissioning Plan (Revision 3).

The BA1 groundwater flow model from the 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020) was improved by decreasing the uniform MODFLOW cell size from ten feet to five feet and through updates of lithology zones within the valley of the BA1 transition zone. The reduction in cell size allows for more accurate analysis of groundwater flow near boundary conditions such as infiltration and extraction trenches. The lithology update improved upon the distribution of lithology zones based on the Environmental Sequence Stratigraphy (ESS) and Porosity Analysis (Burns & McDonnell, 2018), which incorporated the distribution of isolated sand channels within the gulley fill of the BA1 transition zone (Appendix A). After verifying the BA1 groundwater model calibration, the model was used to simulate the resulting groundwater flow field under the proposed nominal extraction and infiltration rates of the current Site Facility Decommissioning Plan (Revision 3). This included simulation of a new infiltration trench GWI-BA1-04 to address the potential for dewatering of the coarse-grained, intra-gully sand deposits between GETR-BA1-01 and GETR-BA1-02. Implementing infiltration trench GWI-BA1-04 raises groundwater levels and provides additional flushing of the pore space in the unconsolidated sediments between GETR-BA1-01 and GETR-BA1-02. Based upon the resulting steady state groundwater flow field from MODFLOW, particle tracking was performed utilizing MODPATH forward particle analysis for a period of 30 years. The results indicate groundwater capture for the BA1 remediation area. Particle tracking also indicates that particles between GETR-BA1-01 and GETR-BA1-02 are either captured by these two infiltration trenches or flushed from the additional flux of GWI-BA1-04 to downgradient extraction wells.

The WAA groundwater flow is primarily based upon the groundwater flow model described by the 2020 Groundwater Flow Model Review (Burns & McDonnell, 2020). After verifying the WAA groundwater model calibration, the model was used to simulate the resulting groundwater flow field under the proposed nominal extraction and infiltration rates of the current Site Facility Decommissioning Plan (Revision 3). Based upon the resulting steady state groundwater flow field from MODFLOW, particle tracking was performed utilizing MODPATH forward particle analysis for a period of 30 years. The results indicate groundwater capture for the WAA remediation area. Revision 0

9.0 REFERENCES

Burns and McDonnell, 2014. Groundwater Flow Model Update, Cimarron Remediation Site. January.

- Burns and McDonnell, 2017a. 2016 Groundwater Flow Model Update, Cimarron Remediation Site. January 25.
- Burns and McDonnell, 2017b. Vertical Distribution of Uranium in Groundwater. May 10.
- Burns and McDonnell, 2018. Environmental Sequence Stratigraphy (ESS) and Porosity Analysis, Burial Area 1, Cimarron Former Nuclear Fuel Production Facility, April 6, 2018
- Driscoll, F.G. 1986. Groundwater and Wells Second Edition. Johnson Filtration Systems Inc., St. Paul Minnesota.
- Faybishenko, B.A., Javandel, I. and Witherspoon, P.A. 1995. Hydrodynamics of the Capture Zone of a Partially Penetrating Well in a Confined Aquifer. Water Resources Research. Volume 31, Issue 4. April.
- ENSR, 2006, Groundwater Flow Modeling Report. October.
- Environmental Properties Management, LLC, 2018. Cimarron Facility Decommissioning Plan, Revision 1. October.
- Harbaugh, A.W., Banta, E.R., Hill, M.C., and McDonald, M.G., 2000. The U.S. Geological Survey Modular Ground-Water Model – User Guide to Modularization Concepts and the Groundwater Flow Process. U.S. Geological Survey Open-File Report 00-92.
- Konikow, L.F., Hornberger, G.Z., Halford, K.J., and Hanson, R.T., 2009, Revised multi-node well (MNW2) package for MODFLOW ground-water flow model: U.S. Geological Survey Techniques and Methods 6–A30,
- Kruseman, G.P., and de Ridder, N.A. 1994. Analysis and Evaluation of Pumping Test Data. International Institute for Land Reclamation and Improvement. The Netherlands.
- Pollock, D.W., 1989. Documentation of a computer program to compute and display pathlines using results from the U.S. Geological Survey modular three-dimensional finite-difference ground-water flow model. U.S. Geological Survey, Open File Report 94-464.

APPENDIX A - 2022 BA1 GROUNDWATER FLOW MODEL LITHOLOGY DISTRIBUTION BY MODFLOW LAYER

· .

.

and the Real of Alternation and an and the second second

•

-

A T R T P AND ANT AND ADDRESS OF A DECK

LTT TA AND AND AND ADDRESS AND A 1944 IN SUC.







(101) CIMARRON RIVER FLOODPLAIN DEPOSITS CLAY/SILT TRACES SAND - KX/KY: 3 (2 & 102) CIMARRON RIVER DEPOSITS - UPPER ALLUVIAL AQUIFER SANDS - KX/KY: 117.5 (104) UPPER GUILTY FILL - KX/KY: 15
(105) LOWER GULLY FILL - KX/KY: 3
(106) INTRA GULLY STREAM DEPOSITS (SAND BODY) - KX/KY: 50
(3) SILT - KX/KY: 0.283
(6,8, & 9) SILTSTONE - KX/KY: 8.43
(4) SANDSTONE A - KX/KY: 40
(7) SANDSTONE B - KX/KY: 5











END MONITOR WELL IN ALLUVIUM MONITOR WELL IN SANDSTONE B MONITOR WELL IN SANDSTONE C MONITOR WELL IN TRANSITION ZONE	(2 & 102) CIMARRON RIVER DEPOSITS - UPPER ALLUVIALAQUIFER SANDS - KX/KY: 117.5 (12) CIMARRON RIVER DEPOSITS - LOWER ALLUVIALAQUIFER SANDS - KX/KY: 352.5 (103) RIVER CLAY PLUG DEPOSITS - KX/KY: 2.77 (104) UPPER GULLY FILL - KX/KY: 15	APPENDIX A BA1 GROUNDWATER FL LAYER 7 - LITHOLOGI SITE FACILITY DECOMMISSIONING	OW MODEL C ZONES PLAN (REVISION 3)
EXTRACTION WELL/SUMP INJECTION WELL GROUNDWATER EXTRACTION TRENCH	(105) LOWER GULLY FILL - KX/KY: 3 (106) INTRA GULLY STREAM DEPOSITS (SAND BODY) - KX/KY: 50 (10) CLAY - KX/KY: 0.5		environmental properties management LLC
BA1 URANIUM CONTOUR (30 UG/L)	(6) SILTSTONE - KX/KY: 8.43		Rev No: 0
TRENCH MODEL CELLS	(7) SANDSTONE B - KX/KY: 5	Preparer: BELOCKWOOD	Date: 9/20/2022
NO FLOW BOUNDARIES	(11) SANDSTONE C - KX/KY: 3	Reviewer: DCLEMENT	Date: 9/20/2022
RIVER BOUNDARY CELLS GENERAL HEAD BOUNDARY CELLS		Coordinate System WGS 1984 Web Mercator Auxiliary Sphe	re







APPENDIX B - ENVIRONMENTAL SEQUENCE STRATIGRAPHY (ESS) AND POROSITY ANALYSIS

Memorandum



Date: April 6, 2018

To: Jeff Lux, P.E.

From: Mike Shultz, PhD

Subject: Environmental Sequence Stratigraphy (ESS) and Porosity Analysis, Burial Area 1, Cimarron Former Nuclear Fuel Production Facility

The ESS analysis described herein includes reviews of existing subsurface data and reformatting of grain size information provided in existing lithologic logs to elucidate trends in grain size. These trends can be interpreted by a stratigrapher in the context of the depositional environments in which aquifer materials were originally laid down. This process yields an updated conceptual site model (CSM) and provides insight into preferential pathways for groundwater migration and contaminant fate and transport. The work products resulting from the ESS analysis consist of:

- 1. A network of cross-sections through the Transition Zone (TZ) and out onto the Cimarron River floodplain (Cross-Sections A-A' through H-H');
- 2. An interpretive isopach map of more permeable deposits within the TZ saturated zone;
- A calculated estimate of the transmissive fraction of the saturated interval within the TZ; and,
- 4. This technical memorandum.

Figure 1A shows the geologic cross-section locations and Figure 1B is provided as a legend for the cross-section symbology. Cross-Sections A-A' through H-H' are included as Figures 2A through 2H. Isopach maps, included as Figures 3A through 3C, show relatively permeable strata thickness with cross-section transects, potentiometric surface, and uranium isopleths, respectively. With the exception of monitor wells 02W29 and 02W46, monitor well water levels presented on the cross-sections were recorded on November 6, 2017. The 02W29 and 02W46 water levels presented on the cross-sections were recorded on July 31, 2017, because water levels recorded at these wells in November 2017 were outside typical historical ranges.

Geologic Setting

The BA-1 area consists of a bedrock bench of Permian-age deltaic channel sands and interbedded claystone (Garber Sandstone) upon which the burial trenches were sited (see Figure IA). An erosional gully partially filled with primarily low-permeability material is present to the north and east of the bedrock ridge, and this gully area has been referred to as the "Transition Zone" between the bedrock escarpment and the sand-rich deposits of the Cimarron River alluvium present to the north of the burial area.

Environmental Sequence Stratigraphy Analysis

The TZ represents a gully eroded into the underlying bedrock which has been partially filled with predominantly fine-grained deposits. The eastern margin of the gully cannot be defined due to the lack of lithologic logs; no borings have been advanced in that area to date. The gully fill

Memorandum (cont'd)



April 5, 2018 Page 2

can be subdivided into a basal clay-rich unit (Lower Gully Fill [LGF]), and an upper silt-rich unit (Upper Gully Fill [UGF]). A relatively sandy deposit marks the base of the UGF unit. An isopach (equal-thickness) map of the sandy zone (relatively permeable strata) at the base of the UGF has been interpreted as part of the ESS analysis (see Figures 3A through 3C). The lateral connectivity of this deposit cannot be fully defined by existing lithologic information. Thus, the isopach map interpreted herein represents a sum of the interpreted permeable thickness and should not necessarily be taken to indicate a "channel" in the sense of a wholly continuous layer of sandy material. However, the consistent position of the sandy deposit at the contact of the UGF and the LGF suggests that it may in fact be hydraulically connected to a certain degree and that a disproportionate percentage of groundwater flow and contaminant mass flux likely occurs within this thin zone. Figure 3B illustrates the groundwater flow directions and geometry of the potentiometric surface as groundwater flows from the upland deposits through the TZ into the alluvial floodplain deposits.

The LGF likely represents slope failure (slump and debris-flow deposits) derived from soil horizons developed atop the bedrock in the immediate vicinity during initial phases of gully development. Flash flood events periodically removed portions of this material in an iterative process of erosion and deposition. With time, as the gully widened and headward erosion of the gully proceeded, the gully captured a greater area and greater volumes of surface water flowed through the gully during rain events. The area of investigation in the TZ was transformed into an alluvial valley and the setting changed from slope failure-dominated deposition to streamflow-dominated deposition. An erosional surface was carved into the underlying LGF by streams, likely during flash flood events. As described above, residual sands at the base of the UGF mark this transition. From this point on, the gully fill is dominated by thin sand channel deposits and silts of the UGF deposited by waning flow after flood events.

Isoconcentration contours of uranium in groundwater are plotted on the isopach map included as Figure 3C. As shown on this figure, the distribution of uranium in the subsurface seems to correlate well with the interpretive isopach map of the permeable material, with the plume extending northwest from the burial trenches, following the gully sand channel deposits through the TZ and out to the alluvial aquifer in the floodplain. In the upper reaches of the gully, the contaminant distribution appears to be controlled by the location and orientation of the burial trenches, the source of contamination. In this area, the permeable TZ materials appear to split into an eastern and a western zone (e.g., Cross-Section G-G'). Contamination appears to be limited to the western permeable unit in this area, likely due to the proximity of the burial trenches. From a CSM perspective, it appears that contaminated groundwater emanating from the BA-1 burial trenches percolates downward through the bedrock and TZ sediments (depending on burial trench location), is discharged into the western arm of the sandy deposits at the UGF/LGF contact, travels northwest within this interval down-gully, and then discharges primarily to the Upper Point Bar (UPB) deposit of the Cimarron River sands.

Memorandum (cont'd)



April 5, 2018 Page 3

Estimate of Transmissive Porosity within BA1 Transition Zone

Boring logs for TZ wells were critically examined as part of the ESS analysis and cross-section creation. Thickness of the sandy unit at the base of the UGF was tabulated for each well and imported into Earth Volumetric Studio[®] (EVS) for calculation of total saturated sand channel volume within the uranium-impacted portion of the TZ (44,458 cubic feet [ft³]). This value was multiplied by an assumed effective porosity for fine silty sand (20%), based on reference values obtained from *Applications of_Environmental Chemistry* – *A Practical Guide for Environmental Professionals*¹, to calculate the transmissive pore volume for saturated sand channel deposits located within the effected BA1 TZ (8,892 ft³).

EVS was also used to calculate the total saturated volume within the uranium-impacted portion of the TZ (511,425 ft³). This volume is comprised of the saturated, uranium-impacted UGF volume (189,137 ft³), the saturated, uranium-impacted LGF volume (277,830 ft³), and the saturated, uranium-impacted sand channel volume (44,458 ft³). The saturated, uranium-impacted UGF and LGF volumes were each multiplied by a conservatively assumed effective porosity for silty-clay (10%), based on reference values¹, to calculate the corresponding transmissive pore volume for these fine-grained deposits – 18,914 ft³ for the UGF and 27,783 ft³ for the LGF. Finally, all three transmissive pore volumes were added together and divided by the bulk volume of impacted, saturated TZ material (511,425 ft³) to calculate the transmissive porosity for the effected BA1 TZ (11%).

The calculation conducted to estimate the transmissive fraction of the saturated interval within the uranium-impacted TZ is presented in Table 1 and two-dimensional (2D) and threedimensional (3D) renderings of the EVS volume calculations are presented on Figures 4 through 7. This work suggests that approximately 9% of the overall TZ saturated thickness is sandy and therefore constitutes a porous interval with the potential to transmit groundwater and contaminant mass. The estimates presented above and in Table 1 assume that the sand channel deposits are in fact permeable and somewhat connected, and that the clay- and silt-rich LGF and UGF are significantly less transmissive.

As stated above, EVS was used to model 3D volumetric 'bodies' for the saturated, uraniumimpacted sand channel, UGF, and LGF TZ deposits. Figure 4 provides a plan view rendering of the volumetric analysis domain. In the horizontal dimension, the northwest domain boundary represents the BA1 TZ/alluvium boundary, the northeast and southeast domain boundaries represent the approximate extent of BA1 uranium groundwater impacts, and the southeast domain boundary (annotated black line) represents the saturated TZ deposit/bedrock interface (at the water table). In the vertical dimension, the water table (depicted as the blue surface on Figure

¹ Weiner, Eugene R, <u>Applications of Environmental Chemistry – A Practical Guide for Environmental</u> <u>Professionals</u>, Taylor & Francis Group, LLC, CRC Press, 2000.

Memorandum (cont'd)



April 5, 2018 Page 4

5) serves as the upper boundary of the volumetric analysis domain, and the basal TZ/bedrock interface (see Figure 5) serves as the lower boundary. Figure 5 provides an orthogonal view of the volumetric analysis domain and the sand channel deposit 'body', with the UGF and LGF deposits hidden in the model. Figure 6 provides the same orthogonal view shown in Figure 5, with the UGF deposits shown and the LGF and sand channel deposits hidden. Finally, Figure 7 provides the same orthogonal view with the LGF deposits shown and the UGF and sand channel deposits hidden.

Cimarron River Deposits

Sand-rich point bar and overlying floodplain deposits of the Cimarron River to the north interfinger with the gully fill deposits (e.g., Cross-Section A-A'). Individual point-bar deposits of the Cimarron River are approximately 5' thick, and in the BA1 investigation area there are two stacked point bar deposits (UPB and Lower Point Bar [LPB]). A sharp grain size increase marks the base of the UPB, and this contact surface is well-displayed in Cross-Sections A-A', B-B', and C-C'. This contact is indicated by a thin zone of increased conductivity in the electrical conductivity (EC) log for 02W32 (Cross-Section D-D'), probably related to a slight increase in clay content in the upper foot of the lower point bar. This contact is also indicated by a color change described in the boring log for 02W32. Depth-discrete sampling at 02W32 (see Cross-Section D-D') suggests that the majority of contaminant mass flux is occurring within the UPB deposits with the sandy unit present at the base of the UGF, suggesting that this is the pathway from the gully fill to the UPB Deposits. The relatively higher concentration within the UPB may be explained by this connection.

Data Gaps and Recommendations

While it is likely that channel sand deposits at the UGF/LGF interface represents the primary pathway for contaminants in the TZ, there are no data related to the vertical distribution of uranium within the TZ gully fill deposits. Attempts at depth-discrete groundwater sampling in TZ material have been unsuccessful due to the low-permeability nature of the gully fill sequence. Vertical profiling of groundwater flow and chemistry within existing wells via dye tracer systems offered by the United States Geological Survey (USGS) and/or BESST, Inc. may provide data to support the CSM presented herein, and the results may be useful in refining the BA1 remedial action implementation plans. In addition, other means of obtaining depth-discrete high-resolution vertical profiling of uranium should be investigated.

Attachments:

Figure 1A:	Cross-Section Location Map
Figure 1B:	Cross-Section Legend
Figures 2A-2H:	Cross-Sections A-A' through H-H'
Figure 3A:	Isopach Map of Relatively Permeable Deposits of the

Memorandum (conr'd)



April 5, 2018 Page 5

	Transition Zone
Figure 3B:	Isopach Map of Relatively Permeable Deposits of the
	Transition Zone with Potentiometric Surface Contours
Figure 3C:	Isopach Map of Relatively Permeable Deposits of the
	Transition Zone with Uranium Isopleth Contours
Table 1:	Transmissive Porosity Calculation for Saturated and
	Contaminated BA1 Transition Zone
Figure 4:	Plan View of Transition Zone Saturated Bedrock and Intra-
	Gully Stream Deposits – Burial Area 1
Figure 5:	Orthogonal View of Saturated Intra-Gully Stream Deposits
	and Bedrock – Burial Area 1
Figure 6:	Orthogonal View of Saturated Upper Gully Fill Deposits
	and Bedrock – Burial Area 1
Figure 7:	Orthogonal View of Saturated Lower Gully Fill Deposits
	and Bedrock – Burial Area 1

cc: John Hesemann Jeff Binder Bill Halliburton

Path: Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2018\ESS Tech Memo\Basemap.mxd COPYRIGHT © 2018 BURNS & McDONNELL ENGINEERING COMPANY, INC. 222600 22200 2000 323200 Service Lever Credita: Source: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Alrbus DS, USDA, USGS, AeroGRID IGN, and the GIS User Community 2095000 2025000 B 0 2095200 (02W21) 21/34 D 02W23 (1367) 02W36 (02W33) (02W24) 02W17 1368 **N35** 02W15 02W45 021 m (02W04) 12W43 A 2119 02W18) 1363 02W53 (02W38) 2015100 2095-100 (02W14) 02W48 TMW-24 F G (1361) (02W11 1365 362 112 C' D. 02W31 364 H m -<u>1</u> 2095500 2095500 H. Ģ B. 370 A. , (1344) 2095800 2095500 2092000 2035000





Cimarron River Floodplain Deposits: Clay, silt, and interbedded fine-grained sand corresponding to floodplain deposits of the Cimarron River. Sands as overbank splays deposited during flood-stages.

Cimarron River Channel Deposits: Fine to coarse-grained, trough cross-bedded sand deposited as point-bars by the Cimarron River. Minor intraclast or extrabasinal conglomerate lags define bases of individual point-bar sequences

Cimarron River Clay Plug Deposits: Clay and silt, some thin sands, deposited in abandoned stretches of Cimarron River channels (oxbow lakes).



Upper Gully Fill: Silt and silty sand with interbedded clayey sand and silty sand deposited as gully-wash by streamflow during flash flood events. Contains minor sand-rich streamflow deposits.



. . .

L __ J

Lower Gully Fill: Clay-rich deposits including gully-wall failure (slump, slide, and debris-flow). Chaotic, may include minor re-worked streamflow deposits.

Intra-gully Stream Deposits: Sand and silty sand deposited by streamflow within gully system.

Estimated Intra-gully Stream Deposits: Sand and silty sand deposited by streamflow within gully system.

Garber Sandstone Bedrock (undifferentiated).

Schematic point bar lateral accretion surface.

Waste Disposal Trench (approximate)





WELL ID

HNW

CLAY SANDY CLAY **GRAVELLY CLAY** SILT SANDY SILT CLAYEY SILT FINE GRAVELY SILT COARSE GRAVELY SILT **CLEAN FINE SAND** SILTY FINE SAND CLEAN MEDIUM SAND CLAYEY MEDIUM SAND MEDIUM SAND WITH FINE GRAVEL GRAVELY SAND WITH COARSE GRAVEL CLEAN COARSE SAND COARSE SAND WITH FINE GRAVEL COARSE SAND WITH COARSE GRAVEL FINE GRAVEL WITH SAND MEDIUM GRAVEL WITH SAND COARSE GRAVEL WITH SAND

mS/m - MILLISIEMENS PER METER

psi - POUNDS PER SQUARE INCH

EC - ELECTRICAL CONDUCTIVITY

HPT - HYDRAULIC PROFILING TOOL

- OBSERVED WATER LEVEL

Figure 1B

CROSS-SECTION LEGEND BURIAL AREA 1 CIMMARON SITE, OKLAHOMA









1023018 BILDING & MANUSI ENGINEED









RIGHT © 2018 BURNS & MCDONNELL ENGINEERING



NOTES

1) Y-AXIS MEASURED IN FEET ABOVE MEAN SEA LEVEL (NORTH AMERICAN VERTICAL DATUM 1983) 2) X-AXIS MEASURED IN FEET 3) ALL CROSS-SECTION SYMBOLS ARE DEFINED ON FIGURE 4-1 4) HORIZONTAL AND VERTICAL SCALES ARE APPROXIMATE 5) SURFACE TOPOGRAPHY IS APPROXIMATE 6) GROUNDWATER ELEVATIONS MEASURED NOVEMBER 6, 2017 7) CORRELATION OF UNITS IS AN INTERPRETATION AND NOT NECESSARILY A DELINEATION OF ACTUAL

EXTENT AND THICKNESS OF EACH INDIVIDUAL UNIT







	LL	50 100 200 Feet Feet Acconnell Engineering. NATES : (INAD 63) STATE PLANE OKLAHOMA NORTH FEET AERIAL PHOTO - 2010 / MAP PRODUCED - 4/8/2018 Zookov
	327600	TONS ARE IN FEET ABOVE MEAN SEA LEVEL TH AMERICAN VERTICAL DATUM 1988) ROM GE-BA1-01 NOT USED IN CONTOURING H CONTOUR LINES APPROXIMATED. LINES ED ON BASAL, SATURATED, INTRA-GULLY FAM DEPOSITS. ROUNDWATER
	322800	IDWATER ELEVATION DATA COLLECTED CH 18. 2015
	· · · · · · · · · · · · · · · · · · ·	
	323000	
	323200	WASTE DISPOSAL TRENCH CROSS-SECTION LINE ESTIMATED THICKNESS OF RELATIVELY PERMEABLE STRATA ZERO EDGE ONE FOOT TWO FEET THREE FEET
	323400	end MONITORING WELL IN TRANSITION ZONE MONITORING WELL IN ALLUVIUM MONITORING WELL IN SANDSTONE A MONITORING WELL IN SANDSTONE B MONITORING WELL IN SANDSTONE C GROUNDWATER FLOW DIRECTION ALLUVIUM DEPOSITS GW CONTOURS
	•	FIGURE 3B FIGURE 3B ISOPACH CONTOURS WITH POTENTIOMETRIC SURFACE BURIAL AREA 1 CIMARRON SITE, OKLAHOMA


2096200	
FIGURE 3C ISOPACH CONTOURS WITH URANIUM ISOPLETHS BURIAL AREA 1 IMARRON SITE, OKLAHOMA	
BURNS MEDONNELL.	
and	
MONITORING WELL IN TRANSITION ZONE	
MONITORING WELL IN ALLUVIUM	
MONITORING WELL IN SANDSTONE A	
MONITORING WELL IN SANDSTONE B	
MONITORING WELL IN SANDSTONE C	
URANIUM CONTOURS IN UG/L	
WASTE DISPOSAL TRENCH CROSS-SECTION LINE	
ESTIMATED THICKNESS OF RELATIVELY	
PERMEABLE STRATA	
ONE FOOT	
TWO FEET	
THREE FEET	

RI and Burn TES : (NAD	ns & McDonni 83) STATE P	ell Engineoring LANE OKLAH	, ONIA NORTH FE	ET
the second	Conception in the local distance	1000 B	Feot	A
50	100		200	N
OLLEC.	TED 2011	THROUG	H 2ND QU	ARTER
I COTO	JRS ARE	BASED C	N 95% UCI	FROM
M DEPO	OSITS.			
ON BA	SAL, SAT	URATED,	INTRA-GU	LLY
CONTO	OUR LINE	S APPRO	XIMATED. L	INES
CROGR	AMS PEF	R LITER		

Table 1 Transmissive Porosity Calculation for Saturated and Contaminated Burial Area #1 Transition Zone Cimarron Environmental Response Trust

Aquifer Zone	Aquifer Material	Bulk Aquifer Volume (ft ³) ¹	Assumed Effective Porosity ²	Transmissive Pore Volume (ft ³)	Total Bulk Aquifer Volume (ft ³) ¹	Total Transmissive Pore Volume (ft ³)	Calculated Effective Porosity
Impacted TZ UGF Deposits	silt and silty sand with interbedded clayey sand and silty sand	189,137	10%	18,914			
Impacted TZ Sand Channel Deposits	sand and silty-sand streamflow deposits	44,458	20%	8,892	511,425	55,588	11%
Impacted TZ LGF Deposits	clay-rich channel wall failure deposits	277,830	10%	27,783			

Notes:

TZ - Transition Zone

UGF -Upper Gully Fill

LGF - Lower Gully Fill

ft³ - cubic feet

¹Calculated using Earth Volumetric Studio[®] software application. Sediment and groundwater depths imported into EVS were taken from TZ borings depicted on cross-sections A-A' through E-E'. Highest groundwater surface elevations depicted on cross-sections were used to determine saturated thickness and volume. Calculated bulk saturated volume within the uranium-impacted portion of the TZ is 609,606 ft³.

²Material-specific effective porosity values based on reference values obtained from *Applications of Environmental Chemistry – A Practical Guide for Environmental Professionals* (Weiner, 2000).

1 of 1









APPENDIX C - 2022 WAA GROUNDWATER FLOW MODEL LITHOLOGY DISTRIBUTION BY MODFLOW LAYER

and a star of a sector of the



APPENDIX C WAA GROUNDWATER FLOW MODEL LITHOLOGY ZONES - MODEL LAYER 1

BURNS MSDONNELL. environmental preprinter management, inc



LEGEND

- MONITOR WELL IN ALLUVIUM
- MONITOR WELL IN SANDSTONE A
- MONITOR WELL IN SANDSTONE B
- MONITOR WELL IN SANDSTONE C
- MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN
- NO FLOW BOUNDARIES
- CONSTANT HEAD BOUNDARIES
- RIVER BOUNDARIES CELLS
- GENERAL HEAD BOUNDARY CELLS UPPER ALLUVIAL AQUIFER SANDS (LITHOLOGY ZONE 5) SANDSTONE (LITHOLOGY ZONE 4)

<u>NOTES</u> 1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988).

0	550	1	,100	Ň
	SCALE IN FEET			\mathbf{V}
			Rev N	<u>lo:</u> 0
Preparer	DHORNE		Date:	10/6/2022
Reviewer	: DCLEMENT		Date:	10/6/2022
Coordinate System NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet				



Z:\Clients\ENS\CERT\ ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figures 4-6 Appendix A

APPENDIX C WAA GROUNDWATER FLOW MODEL LITHOLOGY ZONES - MODEL LAYER 2



LEGEND

- MONITOR WELL IN ALLUVIUM
- MONITOR WELL IN SANDSTONE A
- MONITOR WELL IN SANDSTONE B ...
- MONITOR WELL IN SANDSTONE C
- MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN
- NO FLOW BOUNDARIES

CONSTANT HEAD BOUNDARIES

RIVER BOUNDARIES CELLS

GENERAL HEAD BOUNDARY CELLS

- LOWER ALLUVIAL AQUIFER SANDS
- (LITHOLOGY ZONE 5)

NOTES

1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988).

0 550 SCALE IN FEET	1,100		
	<u>Rev No:</u> 0		
Preparer: DHORNE	Date: 10/6/2022		
Reviewer: DCLEMENT	Date: 10/6/2022		
Coordinate System NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet			



Z:\Clients\ENS\CERT_ClientInfo\Sites\Database\Geospatial\Maps & Dwgs\ArcGIS\BMCD_Files\Arcdocs\2020\2022 - Decommissioning Plan\Figures 4-6 Appendix A

APPENDIX C WAA GROUNDWATER FLOW MODEL LITHOLOGY ZONES - MODEL LAYER 3





LEGEND

- MONITOR WELL IN ALLUVIUM
- MONITOR WELL IN SANDSTONE A -
- --MONITOR WELL IN SANDSTONE B
- MONITOR WELL IN SANDSTONE C ÷.
- MONITOR WELL IN TRANSITION ZONE ACTIVE MODEL DOMAIN NO FLOW BOUNDARIES
- CONSTANT HEAD BOUNDARIES

RIVER BOUNDARIES CELLS

- GENERAL HEAD BOUNDARY CELLS
- SANDSTONE (LITHOLOGY ZONE 4)

NOTES

a sta

1) Groundwater elevations in feet above mean sea level (North American Vertical datum of 1988).

0 550 1 SCALE IN FEET	,100			
	Rev No: 0			
Preparer: DHORNE	Date: 10/6/2022			
Reviewer: DCLEMENT	Date: 10/6/2022			
<u>Coordinate System</u> NAD 1983 StatePlane Oklahoma North FIPS 3501 Feet				





CREATE AMAZING.



Burns & McDonnell World Headquarters 9400 Ward Parkway Kansas City, MO 64114 O 816-333-9400 F 816-333-3690 www.burnsmcd.com